



# higher education & training

Department:  
Higher Education and Training  
**REPUBLIC OF SOUTH AFRICA**

## NASIENRIGLYN

### NASIONALE SERTIFIKAAT BOU- EN STRUKTUURKONSTRUKSIE N6

8 April 2021

Hierdie nasienriglyn bestaan uit 10 bladsye.

**VRAAG 1**

Alle verwysings is uit SANS 10100-1 (2000)

$F_{cu} = 25 \text{ MPa}$

Tabel 2 (4.1.5.1)

$F_y = 250 \text{ MPa}$

Tabel 3 (4.1.5.2)

Breedte = 1,45 meter

Digtheid van die gewapende beton = 2 400 kg/m<sup>3</sup>

1.1 Die lengte van die helling

$$\text{Lengte van helling} = \sqrt{2420^2 + 1760^2} \checkmark$$

$$\text{Lengte van helling} = 2992,32 \text{ mm} \checkmark$$

Berekeninge van las:

Middel:

$$W = \text{Vol} \times \text{digtheid} \times g.a. \times 10^{-3}$$

$$W = 2,992 \times 1,45 \times 0,125 \times 2400 \text{ kg/m}^3 \times 9,81 \times 10^{-3} \checkmark$$

$$W = 12,77 \text{ kN} \checkmark$$

Aantreeë of trappe:

$$W = \text{Vol} \times \text{digtheid} \times g.a. \times 9,81 \times 10^{-3}$$

$$W = \frac{1}{2}bh \times \text{breedte} \times \text{digtheid} \times g.a. \times 9,81 \times 10^{-3}$$

$$W = \frac{1}{2} \times 1,76 \times 0,220 \times 1,45 \times 2400 \text{ kg/m}^3 \times 9,81 \times 10^{-3} \checkmark$$

$$W = 6,609 \text{ kN} \checkmark$$

Totale ontwerp dooie las.

$$1,2 G_n (12,77 \text{ kN} + 6,609 \text{ kN}) = 23,255 \text{ kN} \checkmark \quad (\text{CI 4.2.2.1})$$

Ontwerp aangelegde las

$$1,6 Q_n (2,5 \text{ kN/m}^2 \times 1,45 \text{ m} \times 2,420 \text{ m}) = 14,036 \text{ kN} \checkmark \quad (\text{CI 4.2.2.1}) \quad (8)$$

1.2 Maksimum buigmoment

$$BM_{\max} = \frac{WL}{10} + \frac{WL}{10}$$

$$BM_{\max} = \frac{23,255 \times 2,420}{10} + \frac{14,036 \times 2,420}{10} \checkmark \checkmark$$

$$BM_{\max} = 5,63 + 3,4$$

$$BM_{\max} = 9,02 \text{ kNm} \checkmark \quad (3)$$

1.3

Waarde vir 'K'

(CL 4.3.3.4.1)

$$K = \frac{BM}{fcu b d^2}$$

$$K = \frac{9,02 \times 10^6}{25 \times 1450 \times 100^2} \checkmark$$

$$K = 0,025 < K^1 = 0,156 \checkmark$$

Effektiewe diepte  
(d) = 125 – 25  
Bedecking = 100  
mm

Slegs spanningwapening sal vereis word. ✓

Afstand van die hefarm (Z)

(CL 4.3.3.4.1)

$$Z = d \left\{ 0,5 + \sqrt{0,25 - \frac{K}{0,9}} \right\} \leq 0,95d$$

$$Z = 100 \left\{ 0,5 + \sqrt{0,25 - \frac{0,025}{0,9}} \right\} \leq 0,95 \times 100 \checkmark$$

$$Z = 100(0,971) \leq 0,95 \times 100$$

$$Z = 97,1 \text{ mm} > 95 \text{ mm} \checkmark$$

$$\text{Gebruik } Z = 95 \text{ mm (minste)} \checkmark$$

(6)

1.4

Trekwapening

(CL 4.3.3.4.1)

$$As = \frac{BM}{0,87 fy z}$$

$$As = \frac{9,02 \times 10^6}{0,87 \times 250 \times 95} \checkmark$$

$$As = 436,54 \text{ mm}^2 \checkmark$$

$$\text{Gebruik R10 stawe @ 175 c/c } (As = 449 \text{ mm}^2) \checkmark$$

(3)

1.5

Sekondêre wapening

Tabel 23

(CL 4.11.4.3)

$$\frac{100 As}{AC} = 0,24 \checkmark$$

$$As = \frac{0,24 \times AC}{100}$$

$$As = \frac{0,24 \times 1450 \times 125}{100} \checkmark$$

$$As = 435 \text{ mm}^2$$

$$\text{Gebruik R10 stawe @ 175 c/c } (As = 449 \text{ mm}^2) \checkmark$$

(3)

[23]

**VRAAG 2**

2.1       $L_{\text{effektief}} = 5,75 \text{ mm} \times 0,7$       SABS 0162-1984 (Tabel 19)  
 $= 4025 \text{ mm} \checkmark$

Die deursneeoppervlakte van die staalseksie

$\text{Oppervlakte} = 2(375 \times 16) + (12 \times 450) \checkmark$

$\text{Oppervlakte} = 12000 + 5400$

$\text{Dwarsdeursnee-oppervlakte} = 17400 \text{ mm}^2 \checkmark \quad (3)$

2.2      Die tweede oppervlaktemoment om as x-x

$I_{xx} = 2\left(\frac{bd^3}{12} + al^2\right) + \left(\frac{bd^3}{12}\right)$

$I_{xx} = 2\left(\frac{375 \times 16^3}{12} + 375 \times 16 \times 233^2\right) + \left(\frac{12 \times 450^3}{12}\right) \checkmark \checkmark$

$I_{xx} = 2(128000 \text{ mm}^4 + 325734000 \text{ mm}^4) + 91125000 \text{ mm}^4$

$I_{xx} = 651724000 \text{ mm}^4 + 91125000 \text{ mm}^4$

$I_{xx} = 742849000 \text{ mm}^4 \checkmark \quad (3)$

2.3      Die tweede oppervlaktemoment om as y-y

$I_{yy} = 2\left(\frac{bd^3}{12}\right) + \left(\frac{bd^3}{12}\right)$

$I_{yy} = 2\left(\frac{16 \times 375^3}{12}\right) + \left(\frac{450 \times 12^3}{12}\right) \checkmark \checkmark$

$I_{yy} = 140625000 \text{ mm}^4 + 64800 \text{ mm}^4$

$I_{yy} = 140689800 \text{ mm}^4 \checkmark \quad (3)$

2.4      Die minimum tweede oppervlaktemoment

$\text{Minste tweede oppervlaktemoment} = I_{yy} = 140689800 \text{ mm}^4 \checkmark \quad (1)$

2.5      Die minimum omwentelingstraal

$\text{Minste tweede oppervlaktemoment (I)} = I_{yy} = 121396248 \text{ mm}^4$

$r_{\min} = \sqrt{\frac{I_{yy}}{\text{oppervlakte}}}$

$r_{\min} = \sqrt{\frac{140689800 \text{ mm}^4}{17400 \text{ mm}^2}} \checkmark$

$r_{\min} = 89,9 \text{ mm} \quad (\text{Gebruik } 90 \text{ mm}) \checkmark$

Die slankheidsverhouding

$$L/r = \frac{4025}{90} = 44,72 \quad \checkmark$$

Uit Tabel 17 (SABS 0162-1984)

$$44,72 = 136,28 \text{ MPa} \quad \checkmark \quad (4)$$

2.6 Die maksimum aksiale las

Las = Spanning x oppervlakte

$$\text{Las} = 136,28 \text{ N/mm}^2 \times 17\,400 \text{ mm}^2$$

$$\text{Las} = 2\,371,27 \text{ kN} \quad \checkmark \quad (1)$$

[15]

**VRAAG 3**Alle verwysings is uit SANS 10100-1 (2000)

$$F_{cu} = 25 \text{ MPa}$$

Tabel 2 (4.1.5.1)

$$F_y = 250 \text{ MPa}$$

Tabel 3 (4.1.5.2)

$$\text{Span} = 7,0 \text{ meter}$$

CL 4.3.1.2

Die effektiewe breedte van die balk

Gebruik die mindere van die twee:

(CI 4.3.1.5)

$$1. \text{ Webbreedte} + \frac{l_z}{10}$$

$$= 0,300 + \frac{7}{10} \quad \checkmark$$

Breedte van L-balk = 1 meter breed (1 000 mm)  $\checkmark$

OF

2. Werklike breedte (Nie gegee nie)

Las van die balk

$$\text{Ontwerpde dooie las} = 5,5 \text{ kN/m}^2 \times 1 \times 1,2 \text{ Gn}$$

(CI 4.2.2.1)

$$= 6,6 \text{ kN/m} \quad \checkmark$$

$$\text{Ontwerpde aangelegde las} = 2,75 \text{ kN/m}^2 \times 1,6 \text{ Qn} = 4,4 \text{ kN/m} \quad \checkmark \quad (\text{CI 4.2.2.1})$$

Maksimum buigmoment

$$BM_{max} = \frac{WL^2}{8} + \frac{WL^2}{8}$$

$$BM_{max} = \frac{6,6 \times 7^2}{8} + \frac{4,4 \times 7^2}{8} \quad \checkmark$$

$$BM_{max} = 40,43 + 26,95$$

$$BM_{max} = 67,38 \text{ kNm} \quad \checkmark$$

Afstand van die hefarm (Z)

$$Z = d \left\{ 0,5 + \sqrt{0,25 - \frac{K}{0,9}} \right\} \quad (\text{CL 4.3.3.4})$$

$$Z = 460 \left\{ 0,5 + \sqrt{0,25 - \frac{0,156}{0,9}} \right\} \quad \checkmark$$

$$Z = 460 (0,777)$$

$$Z = 357,42 \text{ mm} \quad \checkmark$$

TrekwapeningNA is in die flens, daarom:

(CL 4.3.3.4.1)

$$As = \frac{BM}{0,87 f_y z}$$

$$As = \frac{67,38 x 10^6}{0,87 x 250 x 357,42} \quad \checkmark$$

$$As = 866,75 \text{ mm}^2 \quad \checkmark$$

Gebruik 2Y25 stawe ( $As = 982 \text{ mm}^2$ )  $\checkmark$ 

[11]

**VRAAG 4**Alle verwysings is uit SANS 10100-1 (2000)

4.1	$F_{cu} = 30 \text{ MPa}$	Tabel 2 (4.1.5.1)
	$F_y = 450 \text{ MPa}$	Tabel 3 (4.1.5.2)

4.1.1 Die netto oppervlakte van die beton

$$\begin{aligned} \text{Netto oppervlakte van die staal} &= 6\left(\frac{\pi d^2}{4}\right) + \left(\frac{\pi d^2}{4}\right) \quad \checkmark \\ &= 6\left(\frac{\pi 25^2}{4}\right) + \left(\frac{\pi 75^2}{4}\right) \quad \checkmark \\ &= 2\ 945,24 + 4\ 417,86 \end{aligned}$$

$$\text{Netto oppervlakte van die staal} = 7\ 363,11 \text{ mm}^2 \quad \checkmark$$

$$\begin{aligned} \text{Netto oppervlakte van beton} &= (650 x 330) - 7\ 363,11 \text{ mm}^2 \\ &= 207\ 136,89 \text{ mm}^2 \checkmark \end{aligned} \quad (4)$$

4.1.2 Die aksiale las

$$N = 0,4 \text{ fcu } A_c + 0,67 \text{ fy } A_s$$

(Klousule 4.7.4.3)

$$N = (0,4 \times 30 \times 207 \ 136,89) + (0,67 \times 450 \times 2 \ 945,24) \checkmark \checkmark$$

$$N = 2 \ 485 \ 642,68 + 887 \ 989,86 \checkmark$$

$$N = 1 \ 597,65 \text{ kN} \checkmark$$

(4)

4.1.3 Deursnee en spasiëring van hegtersHegters:  $\frac{1}{4}$  van die kleinste drukstaaf (Klousule 4.11.4.5.1)

$$\frac{1}{4} \times 25 = 6,25 \text{ mm} \text{ (nie beskikbaar nie)} \checkmark$$

Gebruik minimum R8 hegters.

Spasiëring van die hegters

12 x deursnee van die kleinste drukstaaf

$$12 \times 25 = 300 \text{ mm} \checkmark$$

Gebruik spasiëring van 300 mm.

(2)

4.2 Oppervlakte van die kussingfondament

$$\text{Oppervlakte} = \frac{\Sigma \text{ van Afwaartse laste}}{\text{Opwaartse gronddruk}}$$

$$\text{Oppervlakte} = \frac{1 \ 597,65 \text{ kN} + 750 \text{ kN} + 95 \text{ kN}}{210 \text{ kN/m}^2} \checkmark$$

$$\text{Oppervlakte} = 11,632 \text{ m}^2 \checkmark$$

$$\text{Grootte van die fondament} = \sqrt{11,632 \text{ m}^2}$$

$$3,41 \text{ m} \times 3,41 \text{ m} \checkmark$$

Gebruik basisgrootte van 3,5 m x 3,5 m.  $\checkmark$ 

(4)

[14]

**VRAAG 5**

GEGEE:

H-seksie van  $203 \times 203 \times 53,5 \text{ kg/m}$ :

$$I_{xx} = 49,78 \times 10^{-6} \text{ m}^4$$

$$\text{Oppervlakte} = 6,821 \times 10^{-3} \text{ m}^2$$

$$\frac{H}{2} = \frac{203,9}{2} = 101,95 \text{ mm}$$

Plaat:  $180 \text{ mm} \times 150 \text{ mm} \times 4 \text{ mm HRS}$ 

$$I_{xx} = \left( \frac{BD^3}{12} \right) - \left( \frac{bd^3}{12} \right)$$

$$I_{xx} = \left( \frac{0,18 \times 0,15^3}{12} \right) \text{ minus } \left( \frac{0,172 \times 0,142^3}{12} \right)$$

$$I_{xx} = 50,625 - 41,04 = 9,585 \times 10^{-6} \text{ m}^4 \checkmark$$

$$\text{Oppervlakte} = (0,18 \times 0,15) - (0,172 \times 0,142)$$

$$\text{Oppervlakte} = 2,576 \times 10^{-3} \text{ m}^2 \checkmark$$

$$\text{Totale oppervlakte} = 6,821 \times 10^{-3} \text{ m}^2 + 2,576 \times 10^{-3} \text{ m}^2$$

$$\text{Totale oppervlakte} = 9,397 \times 10^{-3} \text{ m}^2 \checkmark$$

Neutrale as deur die oppervlaktemomente van onder af te gebruik

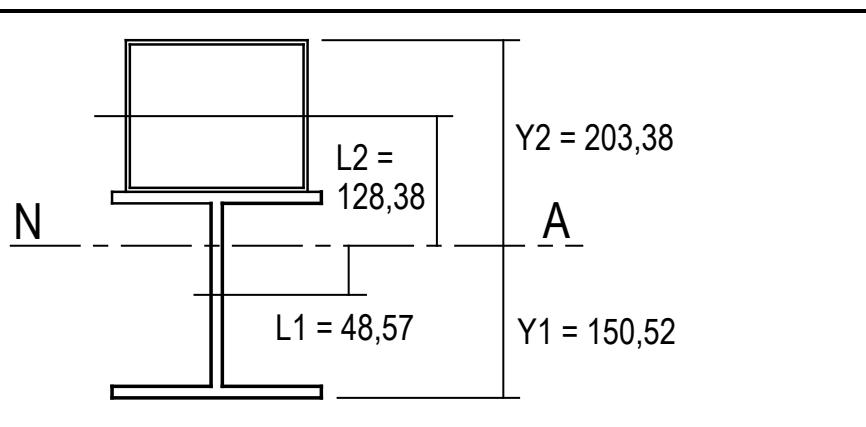
$$9,397 \times 10^{-3} \text{ m}^2 \times Y_1 = (6,821 \times 10^{-3} \text{ m}^2 \times 0,102) + (2,576 \times 10^{-3} \text{ m}^2 \times 0,279)$$

$$0,696 \times 10^{-3} + 0,719 \times 10^{-3} \checkmark \checkmark$$

$$Y_1 = \frac{1,414 \times 10^{-3} \text{ m}^2}{9,397 \times 10^{-3} \text{ m}^2} \checkmark$$

$$Y_1 = 0,15052 \text{ m}$$

$$Y_1 = 150,52 \text{ mm} \checkmark \checkmark$$

Tweede oppervlaktemoment

$$I_{xx \text{ tot}} = (I_{xx \text{ beam}} + al^2) + (I_{xx \text{ RHS}} + al^2)$$

$$I_{xx \text{ Balk}} = (49,78 \times 10^{-6} + 6,821 \times 10^{-3} \times 0,04838^2) \\ = (49,78 \times 10^{-6} + 15,97 \times 10^{-6}) = \underline{65,75 \times 10^{-6} \text{ m}^4} \checkmark \checkmark$$

$$I_{xx \text{ RHS}} = (9,585 \times 10^{-6} + 2,576 \times 10^{-3} \times 0,12838^2) \\ = (9,585 \times 10^{-6} + 42,456 \times 10^{-6}) = \underline{52,041 \times 10^{-6} \text{ m}^4} \checkmark \checkmark$$

$$I_{xx \text{ totaal}} = \underline{117,791 \times 10^{-6} \text{ m}^4} \checkmark$$

Maksimum buigmoment (Buigspanning = 148 MPa)

$$\frac{M}{I} = \frac{f}{y} \quad \text{where} \quad M = \frac{I \times f}{y}$$

$$BM_{\max} = \frac{117,791 \times 10^6 \times 148}{150,52} \quad \checkmark \checkmark$$

$$BM_{\max} = 115,82 \text{ kNm} \quad \checkmark$$

Die UDL Aanvaar eie gewig as 1,25 kN/m

$$BM = \frac{W l^2}{8} + \frac{W l^2}{8}$$

$$115,82 = \frac{1,25 \times 6,5^2}{8} + \frac{W \times 6,5^2}{8} \quad \checkmark$$

$$W = 6,6 + 5,28 W \quad \checkmark$$

$$W = \frac{115,82 - 6,6}{5,28} \quad \checkmark$$

$$W = 20,69 \text{ kN/m} \quad \checkmark$$

[20]

**VRAAG 6**6.1 Die aantal boute vir die deel wat 'B' gemerk is

Force = Eff oppervlakte x spanning x aantal

$$N = \frac{Force}{Pv \times A_{eff}}$$

Spanning =  
Pv = 100 MPa

$$Eff O = \frac{\pi(Dia - 0,9382 P)^2}{4}$$

Klousule 9.5.1

$$Eff O = \frac{\pi(16 - 0,9382 \times 2)^2}{4} \quad \checkmark$$

$$Eff O = 156,67 \text{ mm}^2 \quad \checkmark$$

$$N = \frac{52 \times 10^3}{100 \times 156,67} \quad \checkmark$$

$$N = 3,32 \text{ boute} \quad \checkmark \quad (\text{Gebruik 4 M16-boute}) \quad \checkmark$$

(5)

6.2 Minimum effektiewe lengte van die sveis by 'W'

SABS 0162

$$\text{Dikte van die keel} = \sin 45^\circ \times 5 \text{ mm} \quad \checkmark$$

$$= 3,54 \text{ mm} \quad \checkmark$$

P= 130 MPa  
(CI 10.7.1.2)Bereken die effektiewe lengte:

$$F = P \times Eff O$$

$$F = P \times Keel \times L$$

$$L = \frac{F}{P \times Keel} \quad \checkmark$$

$$L = \frac{66 \times 10^3}{130 \times 3,54} \quad \checkmark$$

$$L = 143,42 \text{ mm} \quad \checkmark$$

(5)

[10]

**VRAAG 7**I-seksie parallel flens $533 \times 210 \times 92,5 \text{ kg/m:}$ 

$$I_{xx} = 553,3 \times 10^{-6} \text{ m}^4$$

$$h/2 = 533,1/2 = 266,5 \text{ mm}$$

$$\text{Buigspanning} = 159 \text{ MPa}$$

Eie gewig van die balk:

$$W = 92,5 \text{ kg/m} \times 9,81 \times 10^{-3}$$

$$W = 0,907 \text{ kN/m} \checkmark$$

$$BM = \frac{WL}{4} + \frac{wl^2}{8} \checkmark$$

$$BM = \frac{150 \times L}{4} + \frac{0,907 \times l^2}{8} \checkmark$$

$$BM = (37,5 L + 0,113 l^2) \text{ kN m}$$

Die spanwydte van die balk:

$$\text{Vanaf } \frac{M}{I} = \frac{f}{y}$$

$$\frac{(37,5 l + 0,113 l^2)10^6}{533,3 \times 10^6} = \frac{159}{266,5} \checkmark$$

$$(9 995,63 l + 30,12 l^2) = 84 794,7 \quad (\text{Deel deur } 30,12) \checkmark$$

$$L = l^2 + 331,86l - 2815,23$$

$$\text{Spanwydte (L)} = \frac{-b \pm \sqrt{b^2 - 4ac}}{2a}$$

$$\text{Spanwydte (L)} = \frac{-331,86 \pm \sqrt{331,86^2 - 4 \times 1 \times 2815,23}}{2 \times 1} \checkmark$$

$$\text{Spanwydte (L)} = \frac{-331,86 \pm 348,41}{2}$$

$$\text{Spanwydte (L)} = 8,3 \text{ meter} \checkmark$$

[7]

**TOTAAL: 100**